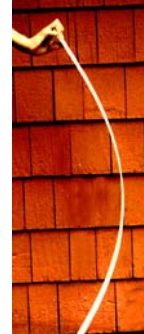


Stability of Columns



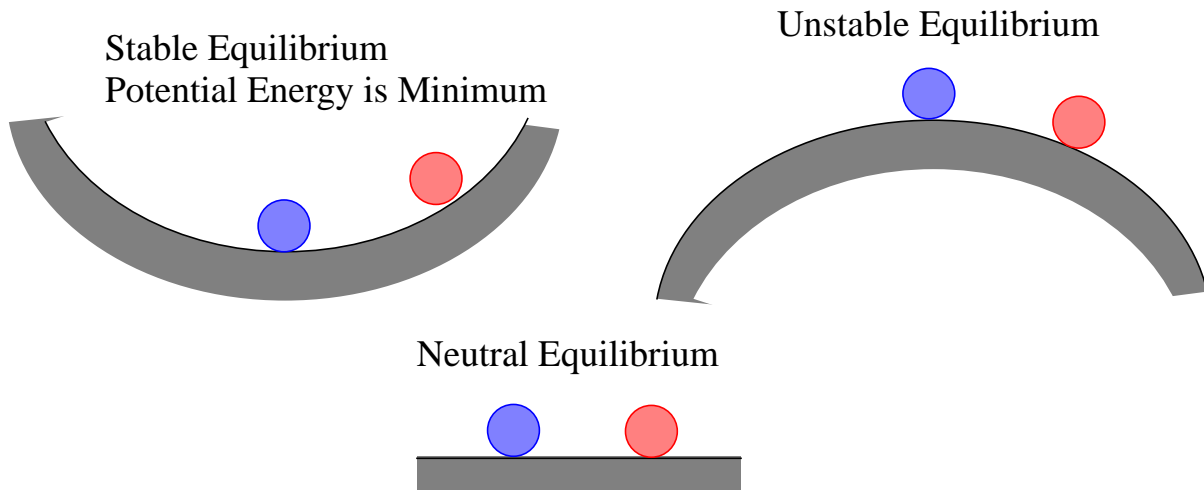
- Bending due to a **compressive** axial load is called **Buckling**.
- Structural members that support compressive axial loads are called **Columns**.
- Buckling is the study of **stability** of a structure's **equilibrium**.

Learning objectives

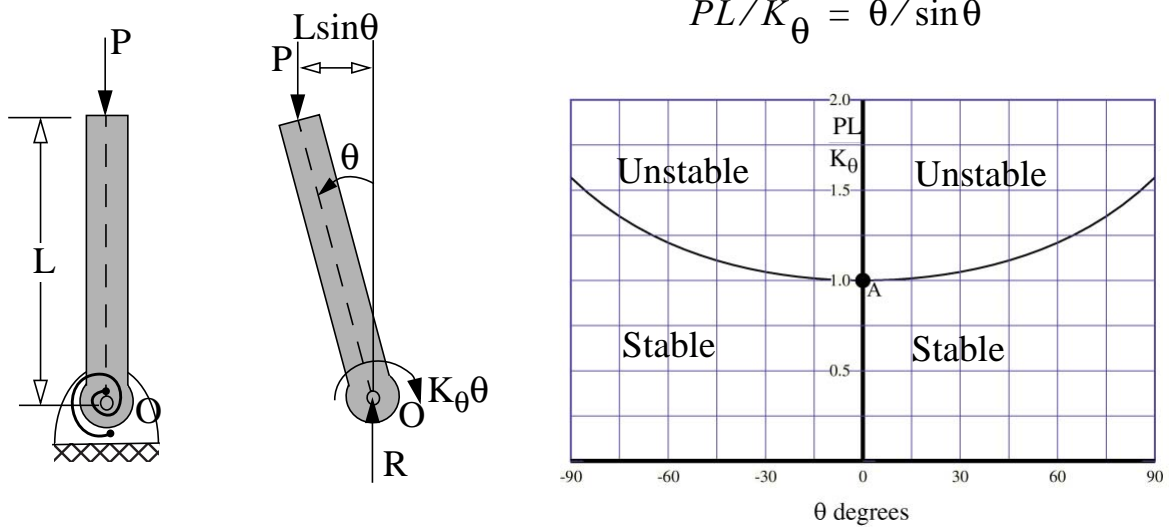
- Develop an appreciation of the phenomena of buckling and the various types of structure instabilities.
- Understand the development and use of buckling formulas in analysis and design of structures.

Buckling Phenomenon

Energy Approach

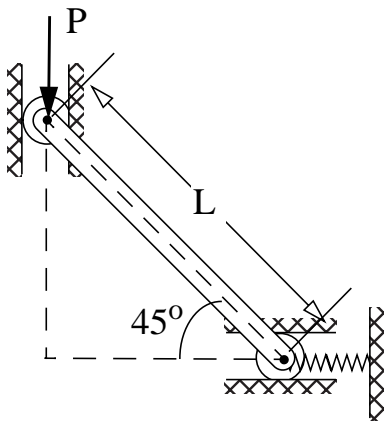


Bifurcation/Eigenvalue Problem

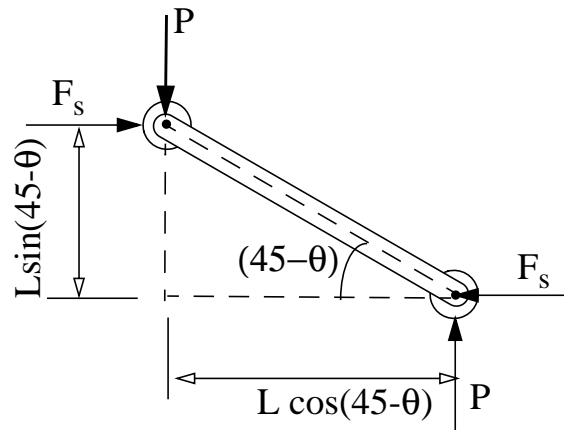


Snap Buckling Problem

$\theta = 0$



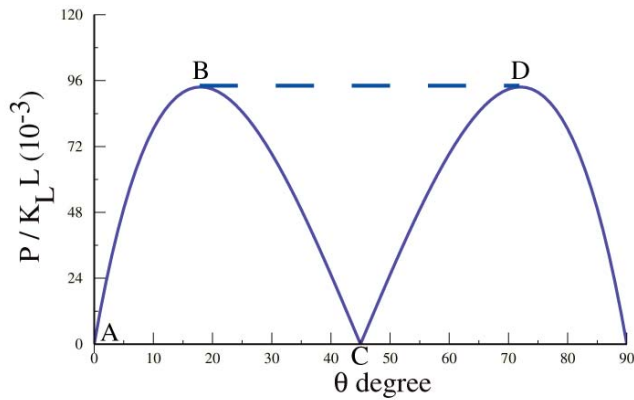
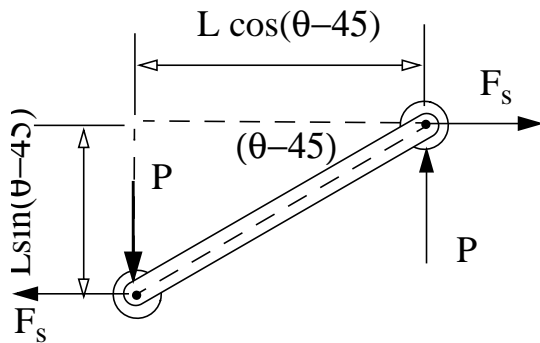
$0 < \theta < 45^\circ$



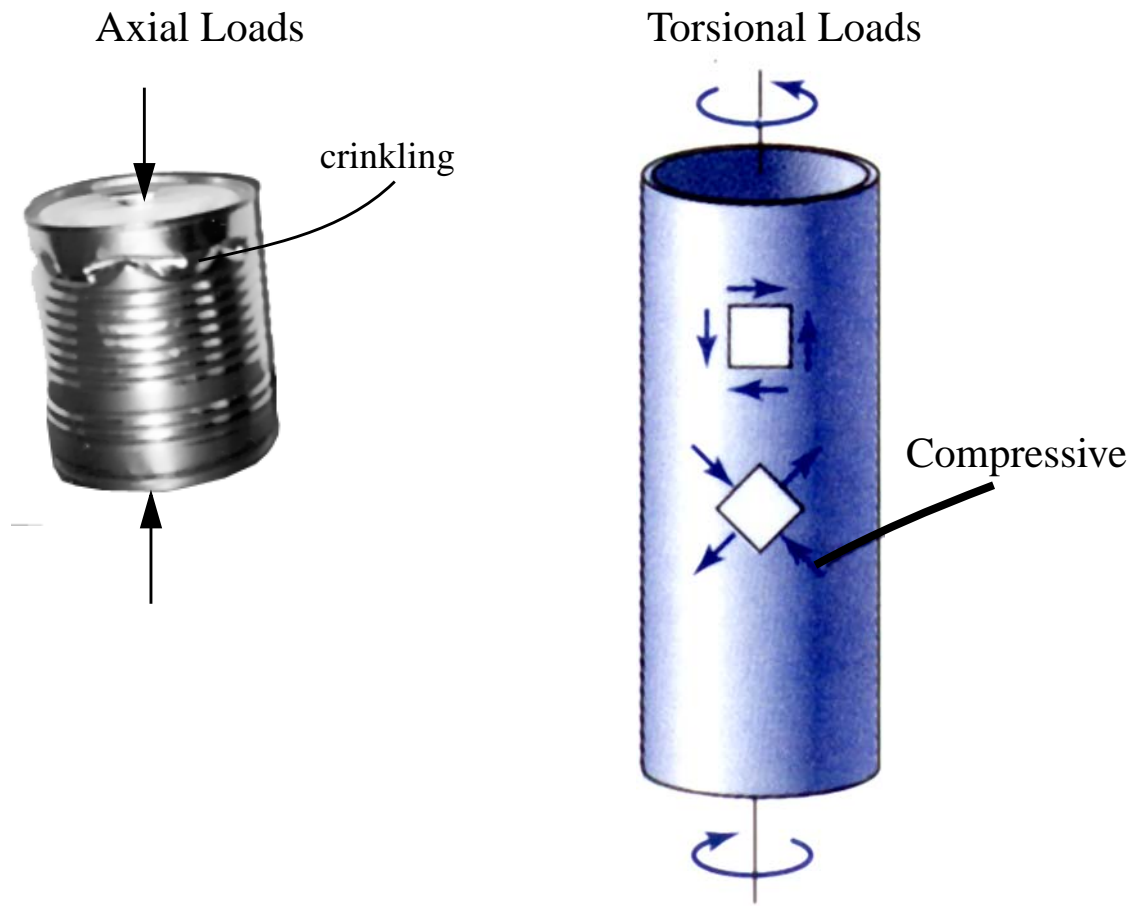
$$\frac{P}{K_L L} = (\cos(45 - \theta) - \cos 45) \tan(45 - \theta) \quad 0 < \theta < 45^\circ$$

$$\frac{P}{K_L L} = (\cos(\theta - 45) - \cos 45) \tan(\theta - 45) \quad \theta > 45^\circ$$

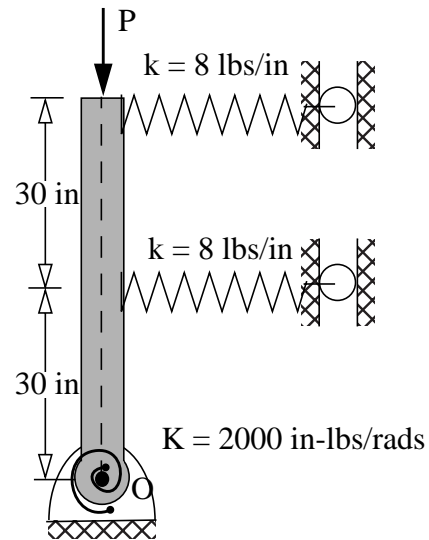
$\theta > 45^\circ$



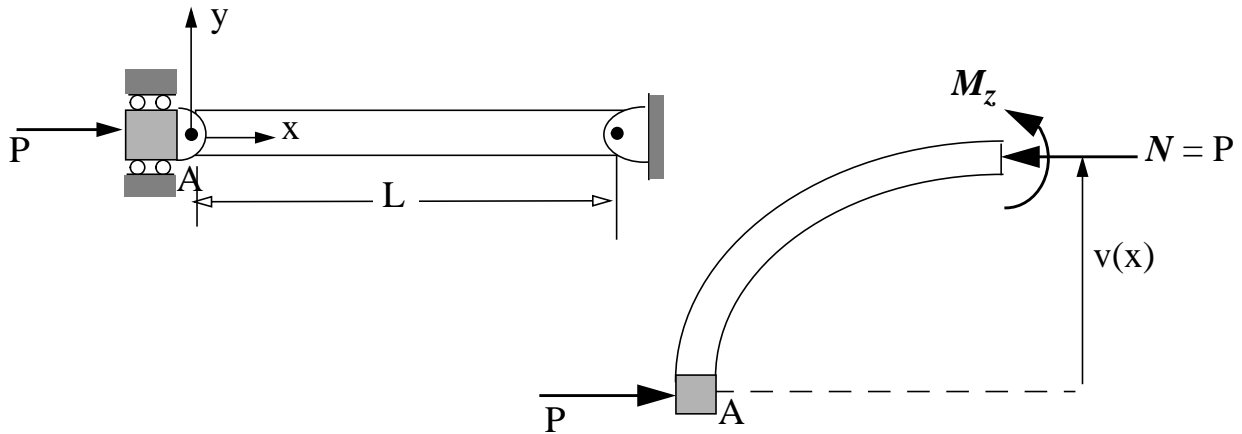
Local Buckling



11.5 Linear deflection springs and torsional springs are attached to rigid bars as shown. The springs can act in tension or compression and resist rotation in either direction. Determine P_{cr} , the critical load value.



Euler Buckling



Boundary Value Problem

$$\text{Differential Equation: } EI_{zz} \frac{d^2 v}{dx^2} + Pv = 0$$

$$\text{Boundary conditions: } v(0) = 0 \quad v(L) = 0$$

Solution

$$\text{Trivial Solution: } v = 0$$

$$\text{Non-Trivial Solution: } v(x) = A \cos \lambda x + B \sin \lambda x$$

$$\text{where: } \lambda = \sqrt{\frac{P}{EI_{zz}}}$$

$$\text{Characteristic Equation: } \sin \lambda L = 0$$

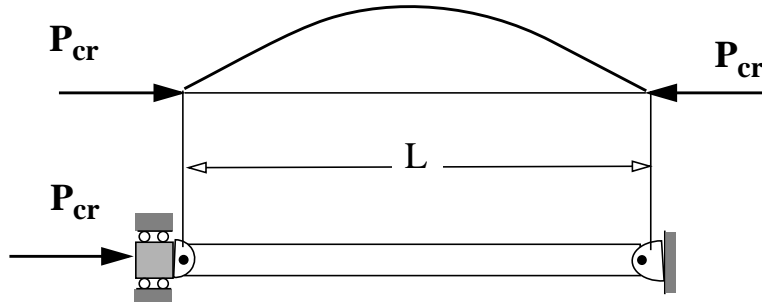
$$P_n = \frac{n^2 \pi^2 EI_{zz}}{L^2} \quad n = 1, 2, 3, \dots$$

$$\text{Euler Buckling Load: } P_{cr} = \frac{\pi^2 EI}{L^2}$$

- Buckling occurs about an axis that has a minimum value of I.

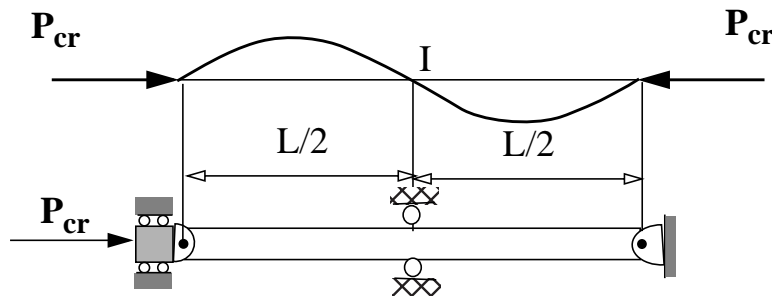
Buckling Mode: $v = B \sin\left(n\pi\frac{x}{L}\right)$

Mode shape 1



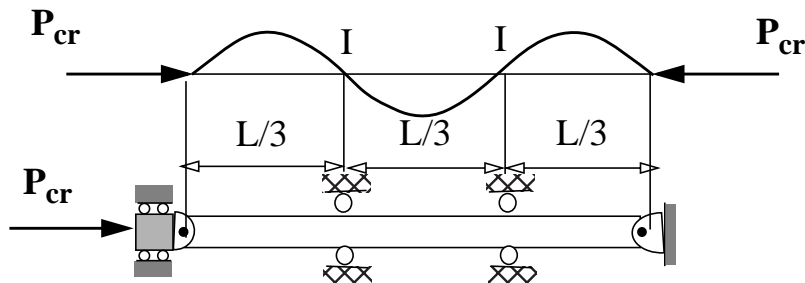
$$P_{cr} = \frac{\pi^2 EI}{L^2}$$

Mode shape 2



$$P_{cr} = \frac{4\pi^2 EI}{L^2}$$

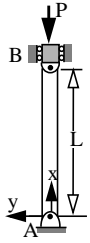
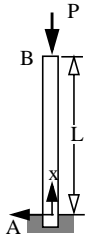
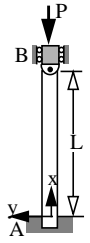
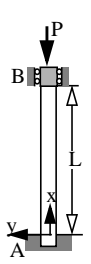
Mode shape 3



$$P_{cr} = \frac{9\pi^2 EI}{L^2}$$



Effects of End Conditions

Case	1. 	2. 	3. 	4. 
Pinned at both Ends	One end fixed, other end free	One end fixed, other end pinned	Fixed at both ends.	
Differential Equation	$EI \frac{d^2 v}{dx^2} + Pv = 0$	$EI \frac{d^2 v}{dx^2} + Pv = Pv(L)$	$EI \frac{d^2 v}{dx^2} + Pv = R_B(L - x)$	$EI \frac{d^2 v}{dx^2} + Pv = R_B(L - x) + M_B$
Boundary Conditions	$v(0) = 0$ $v(L) = 0$	$v(0) = 0$ $\frac{dv}{dx}(0) = 0$	$v(0) = 0$ $\frac{dv}{dx}(0) = 0$ $v(L) = 0$	$v(0) = 0$ $\frac{dv}{dx}(0) = 0$ $v(L) = 0$ $\frac{dv}{dx}(L) = 0$
Characteristic Equation $\lambda = \sqrt{\frac{P}{EI}}$	$\sin \lambda L = 0$	$\cos \lambda L = 0$	$\tan \lambda L = \lambda L$	$2(1 - \cos \lambda L) - \lambda L \sin \lambda L = 0$
Critical Load P_{cr}	$\frac{\pi^2 EI}{L^2}$	$\frac{\pi^2 EI}{4L^2} = \frac{\pi^2 EI}{(2L)^2}$	$\frac{20.13 EI}{L^2} = \frac{\pi^2 EI}{(0.7L)^2}$	$\frac{4\pi^2 EI}{L^2} = \frac{\pi^2 EI}{(0.5L)^2}$
Effective Length— L_{eff}	L	2L	0.7L	0.5L

$$P_{cr} = \frac{\pi^2 EI}{L_{eff}^2}$$

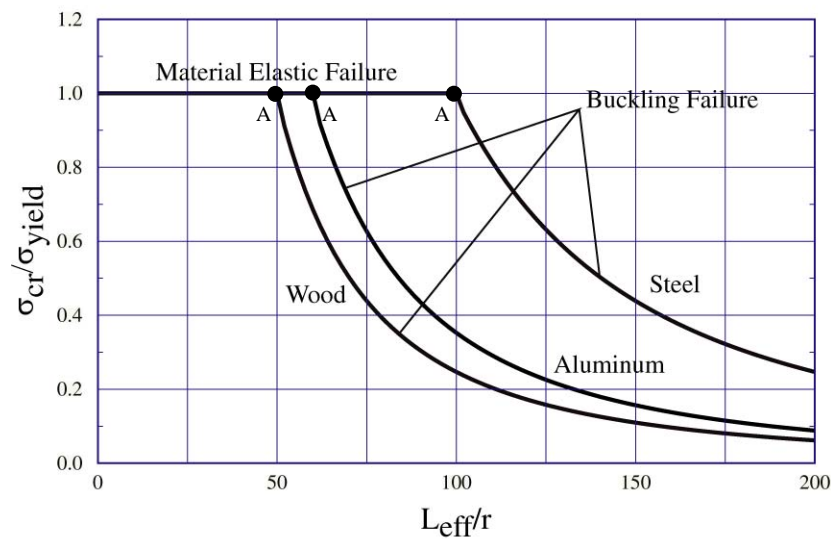
$$\text{Axial Stress: } \sigma_{cr} = \frac{P_{cr}}{A}$$

$$\text{Radius of gyration: } r = \sqrt{\frac{I}{A}}$$

$$\sigma_{cr} = \frac{P_{cr}}{A} = \frac{\pi^2 E}{(L_{eff}/r)^2}$$

Slenderness ratio: (L_{eff}/r) .

Failure Envelopes

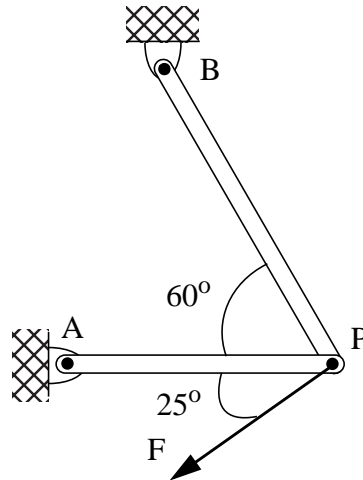


- Short columns: Designed to prevent material elastic failure.
- Long columns: Designed to prevent buckling failure.

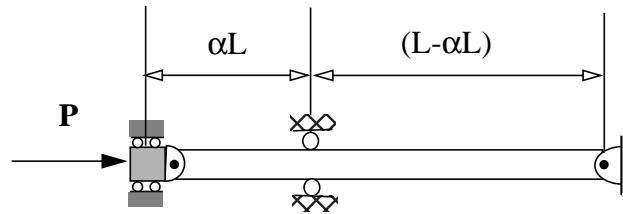
11.13 Columns made from an alloy will be used in a construction of a frame. The cross-section of the columns is a hollow-cylinder of thickness 10 mm and outer diameter of 'd' mm. The Modulus of Elasticity is $E = 200 \text{ GPa}$ and the yield stress is $\sigma_{\text{yield}} = 300 \text{ MPa}$. Table below shows a list of the lengths 'L' and outer diameters 'd'. Identify the long and the short columns. Assume the ends of the column are built in.

L (m)	d (mm)
1	60
2	80
3	100
4	150
5	200
6	225
7	250

11.16 A force $F = 750$ lb is applied to the two-bar structure as shown. Both bars have a diameter of $d = 1/4$ inch, modulus of elasticity $E = 30,000$ ksi, and yield stress $\sigma_{\text{yield}} = 30$ ksi. Bar AP and BP have lengths of $L_{AP} = 8$ inches and $L_{BP} = 10$ inches, respectively. Determine the factor of safety for the two-bar structures.



11.26 Determine the critical buckling in terms of E , I , L , and α . (b) Determine the critical load when $\alpha = 0.5$.



11.40 A hoist is constructed using two wooden bars to lift a weight of 5 kips. The Modulus of Elasticity for wood is $E = 1,800$ ksi and the allowable normal stress 3.0 ksi. Determine the maximum value of L to the nearest inch that can be used in constructing the hoist.

